GENERAL NOTES

GENERAL

- G1. STRUCTURAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE MASSACHUSETTS STATE BUILDING CODE, EIGHTH EDITION, WITH AMENDMENTS TO IBC 2009.
- CONTRACTOR SHALL EXAMINE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR VERIFICATION OF LOCATION AND DIMENSIONS OF CHASES, INSERTS, OPENINGS, SLEEVES, WASHES, DRIPS, REVEALS DEPRESSIONS AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS.
- G3. CONTRACTOR SHALL VERIFY ALL DIMENSIONS ON THE PROJECT.
- G4. OPENINGS IN SLABS OR WALLS LESS THAN 12 INCHES IN DIAMETER ARE GENERALLY NOT SHOWN. OPENINGS SHOWN ON STRUCTURAL DRAWINGS SHALL NOT BE REVISED WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
- G5. OPENINGS IN SLABS, WALLS AND ROOF DECK IN ADDITION TO THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE INCORPORATED INTO THE WORK AS REQUIRED BY THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACT DOCUMENTS.
- G6. DETAILS NOT SPECIFICALLY SHOWN SHALL BE SIMILAR TO THOSE SHOWN FOR THE MOST NEARLY SIMILAR SITUATION AS DETERMINED BY THE ENGINEER.
- G7. UNLESS NOTED ELSEWHERE IN THE CONTRACT DOCUMENTS, REQUIREMENTS NOTED BELOW SHALL APPLY.

FOUNDATIONS

- F1. BASED ON THE REPORT DATED MAY 9, 2016 PREPARED BY MCPHAIL ASSOCIATES LLC., FOOTINGS SHALL BEAR ON UNDISTURBED, NATURAL GLACIAL OUTWAS OR GLACIAL TILL DEPOSIT OR COMPACTED STRUCTURAL FILL PLACED DIRECTLY OVER GLACIAL OUTWASH/TILL. FOOTINGS ARE PROPORTIONED UTILIZING A MAXIMUM ALLOWABLE DESIGN BEARING PRESSURE OF TWO TONS PER SQUARE FOOT.
- BOTTOM OF FOOTING ELEVATIONS SHOWN ON DRAWINGS REPRESENT ESTIMATED DEPTHS AND ARE NOT TO BE CONSTRUED AS LIMITING THE AMOUNT OF EXCAVATION REQUIRED TO REACH BEARING MATERIAL.
- F3. EXTERIOR CONSTRUCTION SHALL BE CARRIED DOWN BELOW FINISHED GRADE TO A MINIMUM DEPTH OF 4 FEET.
- F4. FOOTING EXCAVATIONS ARE TO BE FINISHED BY HAND AND/OR RE-COMPACTED TO 95% OF MAXIMUM DRY DENSITY UNLESS OTHERWISE DIRECTED BY THE GEOTECHNICAL
- THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED ON THE DRAWINGS OR TEST BORINGS. THIS DATA IS INCLUDED ONLY TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND REPRESENT CONDITIONS ONLY AT THE SPECIFIED LOCATIONS AT THE PARTICULAR TIME
- F6. ALL FINISHED FOUNDATION EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE ARCHITECT OR HIS/HER DESIGNATE BEFORE ANY CONCRETE IS PLACED
- F7. UNLESS OTHERWISE NOTED, ALL FOOTINGS AND PILASTERS SHALL BE CENTERED UNDER SUPPORTED MEMBERS.
- DOWELS FROM FOUNDATIONS INTO PIERS, COLUMNS OR WALLS SHALL BE THE SAME SIZE AND SPACING AS THE VERTICAL REINFORCEMENT IN THESE MEMBERS UNLESS OTHERWISE NOTED ON THE DRAWINGS.
- F9. WHERE FOUNDATION ELEMENTS ARE TO HAVE FILL ON BOTH SIDES, EACH SIDE SHALL BE FILLED SIMULTANEOUSLY, MAINTAINING A COMMON ELEVATION.
- PROVIDE DEWATERING AS REQUIRED TO ENSURE THAT NO FOUNDATION CONCRETE IS PLACED IN WATER OR ON FROZEN GROUND
- F8. MAINTAIN ALL EXCAVATIONS IN A DRY CONDITION.

- CONCRETE WORK SHALL CONFORM TO THE LATEST EDITION OF "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318) AND SPECIFICATIONS FOR STRUCTURAL
- C2. CONCRETE SHALL BE CONTROLLED CONCRETE, PROPORTIONED, MIXED AND PLACED UNDER THE SUPERVISION OF AN APPROVED TESTING AGENCY
- C3. CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:
- (A) LEAN CONCRETE. (B) CONCRETE ON STEEL DECK (NORMAL WEIGHT)...... .4000 PS
- (C) ALL OTHER CONCRETE..
- C4. CONCRETE SHALL HAVE THE FOLLOWING NOMINAL DRY DENSITY:
- (A) NORMAL WEIGHT...... 145 PCF
- C5. CONCRETE TO BE EXPOSED TO WEATHER OR FREEZE/THAW ACTION IN THE FINISHED PROJECT SHALL BE AIR-ENTRAINED.
- C6. THE USE OF CONSTRUCTION JOINTS WHERE SHOWN ON THE DRAWINGS IS MANDATORY. OMISSIONS, ADDITIONS OR CHANGES SHALL NOT BE MADE EXCEPT WITH THE SUBMISSION OF A WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE PROPOSED JOINT LOCATIONS FOR APPROVAL OF THE ENGINEER.
- WHERE CONSTRUCTION JOINTS ARE NOT SHOWN, DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP DRAWINGS.
- C8. SIZE OF CONCRETE PLACEMENTS UNLESS OTHERWISE SHOWN SHALL BE AS FOLLOWS

MAXIMUM LENGTH MAXIMUM AREA A. SLABS CAST ON GRADE 60 FEET 3600 SQ. FT. B. WALLS. GRADE BEAMS 30 FEET -----

- C. CONCRETE ON STEEL DECK 60 FEET 3600 SQ. FT. C9. CONCRETE SLABS, INCLUDING SLABS ON STEEL DECK, SHALL BE CAST SO THAT THE SLAB THICKNESS IS AT NO POINT LESS THAN THAT INDICATED ON THE DRAWINGS.
- C10. CONCRETE SLABS SHALL BE CAST ALTERNATELY SO THAT ADJACENT SECTIONS ARE PLACED NO SOONER THAN THREE DAYS APART
- C11. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHOWN OR NOTED. VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRETE WORK SHALL BE MADE AT MIDSPAN OR POINTS OF MINIMUM SHEAR.
- C12. GROUT UNDER COLUMN BASE PLATES AND OTHER BEARING PLATES SHALL BE NON-SHRINK METALLIC GROUT WITH AMINIMUM COMPRESSIVE STRENGTH OF 5000 PSI.
- C13. PIPES OR CONDUITS PLACED IN SLABS ON GRADE SHALL NOT BE PLACED CLOSER THAN 3 DIAMETERS ON CENTERS AND SHALL HAVE AN OUTSIDE DIAMETER LESS THAN 1/4 OF THE SLAB THICKNESS AND BE POSITIONED SO THAT THEY DO NOT INTERFERE WITH SLAB REINFORCEMENT. ALUMINUN CONDUIT SHALL NOT BE PLACED IN CONCRETE. NO CONDUITS OR PIPES SHALL BE PLACED IN COMPOSITE SLABS ON METAL DECK.
- C14. ALL KEYS SHALL BE 2x4 (NOMINAL) UNLESS OTHERWISE SHOWN ON DRAWINGS
- C15. CONCRETE CAST ON SLOPED SURFACES SHALL BEGIN AT THE LOWEST ELEVATION AND CONTINUE MONOLITHICALLY TO THE TOWARDS THE HIGHER ELEVATION UNTIL THE POUR IS COMPLETED.
- C16. ALL EXPOSED EDGES OF CONCRETE MEMBERS SHALL BE CHAMFERED ¾" UNLESS SHOWN OTHERWISE ON THE ARCHITECTURAL DRAWINGS.
- C17. CONCRETE MUST REACH THE FOLLOWING PERCENTAGES OF ITS 28-DAY COMPRESSICE STRENGTH (f'c) BEFORE FORMS OR SHORES MAY BE REMOVED:

WALLS AND BEAM SIDES 20% COLUMNS .. . 40% (1500 PSI MIN.) BEAM BOTTOMS (IF SHORED)........... 70% FLOOR SYSTEMS 85%

- C18. REFER TO ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHES. WHERE FINISH IS NOT SPECIFIED, CONFORM TO REQUIREMENTS OF ACI 301 "SPECIFICATIONS FOR STRUCTURAL
- C19. SEE ARCHITECTURAL DRAWINGS FOR DOOR AND WINDOW OPENINGS, DRIPS, WASHES, REGLETS, CONCRETE FINISHES, MASONRY ANCHORS, AND FOR MISCELLANEOUS EMBEDDED PLATES, BOLTS, ANCHORS, ANGLES, ETC.
- C20. THE PLACEMENT OF SLEEVES, OUTLET BOXES, BOX-OUTS, ANCHORS, ETC., FOR THE MECHANICAL, ELECTRICAL AND PLUMBING TRADES IS THE RESPONSIBILTY OF THE TRADES INVOLVED. HOWEVER, ANY BOX-OUTS NOT COVERED BY TYPICAL DETAILS IN THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED FOR APPROVAL.

REINFORCING

- R1. ALL CONCRETE REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60 EXCEPT WHERE NOTED. ALL REINFORCING TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCING BARS MAY NOT BE WELDED WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
- ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A185. (Fy=65 KSI, MIN) W.W.F. SHALL BE PROVIDED IN FLAT SHEETS. THE FOLLOWING W.W.F. SHALL BE USED FOR AREAS SPECIFIED BELOW UNLESS OTHERWISE SHOWN ON THE DRAWINGS.

SLAB ON GRADE 6x6 - W4.0 x W4.0 SLAB ON METAL DECK 6x6 - W4.0 x W4.0 IN THE MIDDLE OF CONCRETE FILL FOR STAIRS 2x2 - W1.4 x W1.4

- R3. REINFORCEMENT SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS UNLESS OTHERWISE NOTED ON DRAWINGS.
- R4. DETAILING OF REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI-315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED
- PROVIDE AND SCHEDULE WITHTHE SHOP DRAWINGS, ALL NECESSARY ACCESSORIES TO HOLD REINFORCING SECURELYIN POSITION. MINIMUM REQUIREMENTS SHALL BE:

HIGH CHAIRS 4'-0" ON CENTER SLAB BOLSTERS 4'-0" ON CENTER sUPPORT BARS FOR HIGH CHAIRS - NO. 5

- R6. ALL CONTINUOUS REINFORCEMENT SHALL HAVE A MINIMUM LAP AS REQUIRED FOR A CLASS B SPLICE UNLESS NOTED OTHERWISE
- R7. ALL LAPS IN W.W.F. SHALL BE ONE MESH PLUS TWO INCHES AT SPLICES.

REINFORCING (CONTINUED)

R8. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE PROVIDED AS FOLLOWS UNLESS OTHERWISE SHOWN ON THE DRAWINGS: A. SURFACES CAST AGAINST EARTH ..

3/4 INCH

 $\frac{3}{4}$ INCH

- B. FORMED SURFACES EXPOSED TO EARTH OR WEATHER #6 THROUGH #18 BARS.....
- C. FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER SLABS, WALLS, JOISTS . 3/4 INCH BEAMS, COLUMNS. .. 1½ INCHES
- D. SLABS ON GRADE .. 1½ INCHES (FROM TOP OF SLAB). SLABS ON METAL DECK
 - MAXIMUM DEVIATION FROM THESE REQUIREMENTS SHALL BE ± 1/4 INCH FOR SECTIONS 10 INCHES THICK OR LESS; AND ± 1/2 INCH FOR SECTIONS OVER 10 INCHES THICK.
 - R9. ALL HOOKS SHOWN ON DRAWINGS SHALL BE STANDARD HOOKS UNLESS NOTED OTHERWISE.
 - R10. WHERE CONTINUOUS BARS ARE CALLED FOR, THEY SHALL RUN CONTINUOUSLY AROUND CORNERS AND BE LAPPED AT NECESSARY SPLICES, OR HOOKED AT DISCONTINUOUS ENDS. LAP LENGTHS SHALL BE AS GIVEN IN THE SPLICE AND DEVELOPMENT TABLE. LAP BEAM TOP BARS AT MID-SPAN AND BEAM BOTTOM BARS AT SUPPORTS, UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL

- STRUCTURAL STEEL WORK SHALL CONFORM TO "ALLOWABLE STRESS DESIGN SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" (AISC 1989); CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (AISC 1992) AND "STRUCTURAL WELDING CODE - STEEL (AWS D1.1, LATEST EDITION).
- STRUCTURAL STEEL SHALL BE DETAILED IN ACCORDANCE WITH "DETAILING FOR STEEL CONSTRUCTION (AISC)" AND WHERE REQUIRED, DESIGNED IN ACCORDANCE WITH CITED
- STRUCTURAL STEEL DETAILS NOT SPECIFICALLY SHOWN SHALL BE TAKEN AS BEING SIMILAR TO THOSE SHOWN FOR THE MOST NEARLY SIMILAR CONDITION AS DETERMINED BY THE
- STRUCTURAL STEEL SHALL BE NEW STEEL CONFORMING TO THE FOLLOWING:

ASTM A992 GRADE 50 ($F^{\gamma} = 50 \text{ KSI}$) BASE PLATES SHALL CONFORM TO ASTM A36 (F = 36 KSI) HSS SHAPES - ASTM A500 GRADE B ($F_{\gamma} = 46 \text{ KSI}$) PIPES - ASTM A501 OR ASTM A53, TYPE E OR S GRADE B

> ANCHOR BOLTS, LEVELING PLATES, OR BEARING PLATES SHALL BE LOCATED AND BUILT INTO CONNECTING WORK, PRESET BY TEMPLATES OR SIMILAR METHODS. PLATES SHALL BE SET IN FULL BEDS OF NON-SHRINK GROUT.

BOLTED CONNECTIONS SHALL BE AS FOLLOWS:

HIGH STRENGTH BOLTS - ASTM A325

ANCHOR BOLTS - ASTM F1554

MINIMUM BOLT DIAMETER -3/4". TWO BOLTS MINIMUM STANDARD, OVERSIZED, OR HORIZONTAL SHORT SLOTTED HOLES IN WEBS OF BEAMS.

- SHEAR CONNECTIONS FOR MOMENT CONNECTED MEMBERS SLIP CRITICAL HIGH STRENGTH BOLTS IN SINGLE SHEAR. SHEAR CONNECTIONS FOR OTHER MEMBERS - SIMPLE SHEAR CONNECTIONS WITH EITHER FRICTION TYPE HIGH STRENGTH BOLTS IN SINGLE SHEAR OR BEARING TYPE HIGH STRENGTH BOLTS (THREADS INCLUDED IN SHEAR PLANE) IN SINGLE OR DOUBLE SHEAR. SIMPLE SHEAR CONNECTIONS SHALL BE CAPABLE OF END ROTATION PER AISC REQUIREMENTS FOR "UNRESTRAINED MEMBER".
- S7. WELDED CONNECTIONS SHALL BE MADE BY APPROVED CERTIFIED WELDERS USING FILLER METAL CONFORMING TO E70XX OR F7X-EXXX WITH LOW HYDROGEN.
- S8. WELDS SHALL DEVELOPE FULL STRENGTH OF MATERIALS BEING WELDED, UNLESS OTHERWISE NOTED, EXCEPT THAT FILLET WELDS SHALL BE A MINIMUM OF 🛚 🔏 ".
- BEAM CONNECTIONS, UNLESS BEAM REACTION IS NOTED, SHALL PROVIDE CONNECTION CAPACITY AS FOLLOWS: NON-COMPOSITE BEAMS: SUPPORT A REACTION "R" EQUAL TO ½ THE TOTAL UNIFORM CAPACITY OF BEAM FOR A GIVEN SHAPE, SPAN, AND GRADE OF STEEL PER "MAXIMUM TOTAL
- UNIFOR LOAD TABLES (LRFD)" AISC "MANUAL OF STEEL CONSTRUCTION". (B) COMPOSITE BEAMS SUPPORT A REACTION "R c" = MULTIPLIER x "R" (AS DEFINED ABOVE) $R^c = 1.50 \times R$ (FOR BEAM DEPTH GREATER THAN 21")
- R° = 2.00 x R (FOR BEAM DEPTH GREATER THAN 14" BUT LESS THAN 21") R° = 2.25 x R (FOR BEAM DEPTH GREATER THAN 8" BUT LESS THAN 14") ADD TO "R" OR "Re" THE LOADS OR REACTIONS OF MEMBERS SUPPORTED BY THE BEAM NEAR SUPPORTS AND/OR THE VERTICAL COMPONENTS OF FORCE IN DIAGONAL BRACING MEMBERS FRAMING INTO BEAM.
- S10. ENDS OF COLUMNS AT SPLICES AND AT OTHER BEARING CONNECTIONS SHALL BE "FINISHED TO BEAR" TO COMPLETE TRUE BEARING.
- S11. PROVIDE STIFFENERS "FINISHED TO BEAR" UNDER ALL LOAD CONCENTRATIONS ON SUPPORTING MEMBERS, OVER COLUMNS AND WHERE SHOWN ON DRAWINGS.
- S12. PROVIDE TEMPORARY ERECTION BRACING AND SUPPORTS TO HOLD STRUCTURAL STEEL FRAMING SECURELY IN POSITION. SUCH TEMPORARY BRACING AND SUPPORTS SHALL NOT BE REMOVED UNTIL PERMANENT BRACING HAS BEEN INSTALLED AND CONCRETE FOR FLOOR SLABS HAVE ATTAINED 75% OF SPECIFIED CONCRETE STRENGTH.
- S13. STRUCTURAL STEEL MEMBERS SHALL BE FIREPROOFED PER SPECIFICATIONS. S14. FIELD CUTTING OF STRUCTURAL STEEL OR ANY MODIFICATIONS OF STRUCTURAL STEEL SHALL NOT BE MADE WITHOUT PRIOR APPROVAL BY ARCHITECT FOR EACH SPECIFIC CASE.
- S15. STRUCTURAL STEEL MEMBERS SHALL BE TRUE AND PLUMB BEFORE CONNECTION ARE FINALLY BOLTED OR WELDED. S16. FIELD MEMBERS AND CONNECTIONS EXPOSED TO WEATHER SHALL BE GALVANIZED.
- S17. CAMBER SHALL BE COLD-FORMED PROCESS IN CONFORMANCE WITH AISC SPECIFICATION AND TOLERANCE.

STEEL JOISTS

- J1. DESIGN, DETAIL, FABRICATE AND ERECT ALL STEEL JOISTS IN ACCORDANCE WITH THE LATEST "STANDARD SPECIFICATIONS" OF THE STEEL JOIST INSTITUTE.
- J2. DESIGN JOISTS FOR UPLIFT.
- J3. WELD JOISTS TO STEEL SUPPORTS WITH TWO FILLET WELDS AT EEACH END, 2" LONG, EACH SIDE OF JOIST BEARING PLATE
- J4. ALL JOISTS SHALL BE THOROUGHLY INSPECTED DURING FABRICATION T O ENSURE COMPLIANCE WITH CODES AND GOOD WORKMANSHIP.
- J5. HANDLING AND ERECTION SHALL BE DONE WITH CARE. NO DUMPING ON THE GROUND WILL BE PERMITTED.
- J6. HANGERS FOR DUCTS, PIPES, UNITS, ETC. MUST BE ATTACHED TO JOISTS AT PANEL POINTS ONLY. (SEE TYPICAL DETAILS)

STEEL DECK

D1. STEEL ROOF DECK SHALL BE GALVANIZED AND SHALL CONFORM TO ASTM A611, GRADE C

100 PSF

- D2. FABRICATE DECK UNITS IN LENGTHS TO SPAN THREE OR MORE SUPPORT SPACINGS.
- D3. GENERAL CONTRACTOR SHALL COORDINATE SIZE AND LOCATION OF FLOOR AND ROOF OPENINGS WITH ARCHITECTURAL AND MEP DRAWINGS

DESIGN LOADS

- LO. LOADS SHALL CONFORM TO THE REQUIREMENTS OF THE MASSACHUSETTS STATE BUILDING CODE, 8TH EDITION AMENDMENTS TO IBC 2009
- L1. FLOOR LIVE LOADS

- 50 PSF CLASSROOMS CORRIDORS ABOVE FIRST FLOOR 80 PSF FIRST FLOOR CORRIDORS 100 PSF
- PARTITION LOAD
- OFFICES 50 PSF 20 PSF L2. ROOF LIVE LOADS

STAIRS AND EXITWAYS

- GROUND SNOW LOAD: Pg = 55 PSF FLAT SNOW LOAD: Pf = 0.7 (Ce) (Ct) (I) Pg PLUS DRIFTING WHERE APPLICABLE
- EXPOSURE FACTOR: Ce = 0.9

THERMAL FACTOR: Ct = 1.0IMPORTANCE FACTOR: I = 1.1

WIND EXPOSURE CATEGORY: C

- L3. WIND LOADS BASIC WIND SPEED (THREE-SECOND GUST): V = 100 mph IMPORTANCE FACTOR: (Iw) = 1.15 OCCUPANCY CATEGORY: III
- PRESSURE 21 PSF L.4 SEISMIC

IMPORTANCE FACTOR: I = 1.25, OCCUPANCY CATEGORY III MAPPED SPECTRAL RESPONSE ACCELERATIONS: S = 0.29, S1 = 0.071 SITE CLASS: D SPECTRAL RESPONSE COEFFICIENTS: S DS = 0.272, SD1 = 0.114 SEISMIC DESIGN CATEGORY: B BASIC SEISMIC FORCE RESISTING SYSTEM: CONCENTRIC BRACED FRAMES SEISMIC RESPONSE COEFFICIENT:Cs = .097 EFFECTIVE SEISMIC WEIGHT W = 117 KIPS DESIGN BASE SHEAR: V = CsW = 12 KIPS RESPONSE MODIFICATION FACTOR: $R = 3\frac{1}{2}$

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD

ABBREVIATION ALLOWABLE STRESS DESIGN KIP (1000 POUNDS) ALTERNATE AASHTO AMERICAN ASSOCIATION OF STATE LEFT END HIGHWAY & TRANSPORTATION OFFICIALS LIGHTWEIGHT LIGHTWEIGHT CONC AMERICAN CONCRETE INSTITUTE LWC AMERICAN INSTITUTE OF ARCHITECTS LRFD LOAD & RESISTANCE FACTOR DESIGN AMERICAN INSTITUTE OF STEEL LOCATION CONSTRUCTION LONG LEG VERTICAL AMERICAN INSTITUTE OF TIMBER LOW POINT CONSTRUCTION LOWER LAYER **ARCHITECTURAL** AMERICAN SOCIETY OF CIVIL ENGINEERS MANUFACTURER AMERICAN SOCIETY FOR TESTING MASONRY MATERIALS MATL MATERIAL AMERICAN WELDING SOCIETY MECH **MECHANICAL** MEP **ANCHOR BOLT** MECHANICAL, ELECTRICAL, PLUMBING AT RATE OF MEZZ MEZZANINE MID MIDDLE BALANCE MID-PT MIDPOINT NFoPA NATIONAL FOREST PRODUCTS ASSOCIATION BEARING BLOCK NEAR FACE B OR BOT BOTTOM NORMALWEIGHT CONCRETE BOTTOM EACH WAY NOT IN CONTRACT BRACKET

ABBREVIATIONS

AISC

ASCE

ASTM

FND

HEF

BRKT NOT TO SCALE BLDG BUILDING NO OR # CIP CAST-IN-PLACE OSHA OCCUPATIONAL SAFETY & HEALTH CENTER OF GRAVITY **ADMINISTRATION** CTRD CENTERED ON CENTER CLEAN OUT OPENING CENTERLINE OPPOSITE HAND **OUTSIDE DIAMETER** CLEAR COL COLUMN PILE CAP CONC CONCRETE CMU CONCRETE MASONRY UNIT PLATE POINT CRSI CONCRETE REINFORCING STEEL INST POLYVYNYL CHLORIDE CONN CONNECTION PORTLAND CEMENT ASSOCIATION CONST CONSTRUCTION CONST JT OR G CONSTRUCTION JOINT POST TENSIONED POUNDS PER SQUARE FOOT CONT CONTINUOUS POUNDS PER SQUARE INCH CONTROL JOINTS PRECAST CONCRETE DEPR DEPRESSION PRECAST CONCRETE INSTITUTE DET PTW PRESSURE TREATED WOOD DEVELOPMENT LENGTH PRESTRESSED DIA OR Ø

DIAMETER DIMENSION REFERENCE DIRECTION REINF REINFORCE or REINFORCEMENT/ING DITTO REM DWLS DOWELS REMAINDER RETG RETAINING DOWN RETURN DRAWING RIGHT END EACH ROOF DRAIN EACH END SECTION EACH FACE SHEAR CONNECTOR EACH SIDE ELEVATION SHORT LEG VERTICAL FI FVATOR **EPOXY COATED** SOG SLAB ON GRADE **EQUAL** SPA SPACES **EXP BOLT** EXPANSION BOLT SPECS **SPECIFICATIONS** SPLICE LENGTH EXP JT EXPANSION JOINT EXTERIOR SQUARE EXT STAINLESS STEEL FAR FACE STD STANDARD FEET OR FOOT STEEL DECK INSTITUTE FIN FL STEEL JOIST INSTITUTE FINISHED FLOOR STEP FOOTING **FIREPROOF** FLOOR STIFF STIFFENER FLOOR DRAIN STR STRUCTURAL SUMP PIT FTG FOOTING FOUNDATION SUP SUPPORT SYM SYMMETRICAL **GALV** GALVANIZED TEMP **TEMPERATURE** GAGE OR GAUGE THK THICK OR THICKNESS GRADE

THRD GRADE BEAM THREADED TIE BEAM HEIGHT TIMBER HIGH POINT **TOP & BOTTOM** HIGH STRENGTH H OR HORIZ TOC TOP OF CONCRETE HORIZONTAL TOS HORIZONTAL EACH FACE TOP OF STEEL TOW TOP OF WALL HORIZONTAL INSIDE FACE TYP HOF TYPICAL HORIZONTAL OUTSIDE FACE UNO **UNLESS NOTED OTHERWISE** UPPER LAYER INCLUSIVE OR INCLUDING **UNDERWRITERS LABORATORIES** INFO U.L. INSIDE DIAMETER V OR VERT INSUI INSULATION VERTICAL EACH FACE INTERNATIONAL BUILDING CODE VEF VERTICAL INSIDE FACE INVERT VOF VERTICAL OUTSIDE FACE **JOINT** WPG WATERPROOFING WELDED WIRE FABRIC **WWPA** WESTERN WOOD PRODUCTS ASSOCIIATION

WORKING POINT

WATERSTOP

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SCALE

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BID DOCUMENTS